## ME1315

## CIVIL ENGINEERING Paper - 2

Sl.No. 210341

Duration: 150 Minutes

Series



Max. Marks : 300

## **INSTRUCTIONS TO CANDIDATES**

- 1. Please check the Test Booklet immediately on opening and ensure that it contains all the 150 multiple choice questions printed on it.
- Separate Optical Mark Reader (OMR) Answer Sheet is supplied to you along with the Question Paper Booklet. The OMR Answer sheet consists of two copies i.e., the Original Copy (Top Sheet) and Duplicate Copy (Bottom Sheet). The OMR sheet contains Registered Number/Hall Ticket Number, Subject/ Subject Code, Booklet Series, Name of the Examination Centre, Signature of the Candidate and Invigilator etc.,
- 3. If there is any defect in the Question Paper Booklet or OMR answer sheet, please ask the invigilator for replacement.
- 4. Since the answer sheets are to be scanned (valued) with Optical Mark Scanner system, the candidates have to USE BALL POINT PEN (BLUE/BLACK) ONLY for filling the relevant blocks in the OMR Sheet including bubbling the answers. Bubbling with Pencil / Ink Pen Gel Pen is not permitted in the examination.
- 5. The Test Booklet is printed in four (4) Series, viz. A or B or C or D. The Series A or B or C or D is printed on the right-hand corner of the cover page of the Test Booklet. Mark your Test Booklet Series in Part C on side 1 of the Answer Sheet by darkening the appropriate circle with Blue/Black Ball point pen.

Example to fill up the Booklet series

If your test Booklet Series is A, please fill as shown below:









SEAL

1)	If $\theta$ be the angle of slope and the lengt to be applied per chain length is	h of	chain is 30m, t	hen	the correction
	(1) $30 (1-\sec \theta) \text{ m}$	(2)	$30 (1 - \cos \theta)$	m	
-		1	30 (cos θ - 1) n		
2)	Which one of the following angles ca cross staff?	in be	e set out with t	he h	elp of French
	(1) 180° (2) 90° (4)	(3)	45°	(4)	any angle
3)	A lamp at the top of a light house is station at sea level, the distance of the light of the light house.				
	(1) 2.019 m (2) 0.57 m	(3)	20.19 m	(4)	6.057 m
4)	The line Joining points of equal dip is of	calle	d		
300	(1) Isoclinic lines (2) Aclinic lines (			(4)	Agonic lines
5)	If the forebearing of a line AB is 50 included angle between the lines is	° an	d that of line B	C is	20°, then the
	$(1) 70^{\circ}$ $(2) 120^{\circ}$ $($	(3)	220°	(4)	150°
6)		(2)	os each other, it is Overhanging cl Vertical cliff		cates
7)	In theodolite survey, the telescope is sa  (1) Vertical circle is to the right of the original is up				f the telescope
	(2) Vertical circle is to the right of the or is down	bse	rver and the bubb	ole o	f the telescope
	(3) Vertical circle is to the left of the o	bsei	rver and the bubb	ole o	f the telescope
	(4) Vertical circle is to the left of the oris down	bsei	ever and the bubb	ole o	f the telescope
8)	In Geographic information system, line	in p		is ch	aracteristic of
		(2)	Raster overlay	MY BY	
	(3) Intersecting operation	(4)	Vector overlay		
	5				[P.T.O.

9)	If the RL of a BM is 100m, the back sight is 1.415m and the foresight is 1.670m, the RL of the forward station is
	(1) 101.670m (2) 101.415m (3) 99.745m (4) 98.585m
10)	In functional classification of highways, which one of the following highway type have highest mobility and less accessibility
	<ol> <li>National Highways</li> <li>Major District roads</li> <li>State Highways</li> <li>Street and Village Roads</li> </ol>
11)	Webster's equation for computing saturation flow rate in signal design (S= saturation flow rate PCU/hour; W = carriage way width in meters.)
	(1) $S=225 \text{ W PCU/hour}$ (2) $S=525 \text{ W PCU/hour}$
	(3) $S = 550 \text{ W PCU/hour}$ (4) $S = 250 \text{ W PCU/hour}$
12)	For the calculation of stopping distance, the longitudinal friction co-efficient values of have been recommended by Indian Roads Congress.
	(1) 0.35 to 0.40 (2) 0.15 to 0.20 (3) 0.40 to 0.45 (4) 0.45 to 0.50
13)	In total reaction time of the driver, the time required for the sensations received by the eyes/ears to be transmitted to the brain through the nervous system and spinal chord is called
	(1) Intellection time (2) Emotion time
	(3) Volition time (4) Perception time
14)	Calculate the value of lag distance in SSD for a highway with a design speed of 65 kmph?
	(1) 45.18 m (2) 36.14 m (3) 32.50 m (4) 451.80 m
15)	The psychological widening of pavement is calculated using which one of the following formula (take V= speed of vehicle in kmph; R = radius of the curve in m)
	(1) $\frac{V^2}{9.5\sqrt{R}}$ (2) $\frac{V}{9.5\sqrt{R}}$ (3) $\frac{V}{9.5R}$ (4) $\frac{V^3}{9.5R^2}$
16)	Find out the rate of change of centrifugal acceleration for a design speed of 75 kmph, using IRC recommended formula.
	(1) $0.63 \text{ m/sec}^3$ (2) $0.53 \text{ m/sec}^3$ (3) $0.48 \text{ m/sec}^3$ (4) $0.73 \text{ m/sec}^3$

17)	Estimate the theoretical capacity of a traffic lane with traffic flow stream speed of 50 kmph (take average centre to centre spacing of vehicles as 10 m)  (1) 500 vehicles /hour/lane  (2) 5000 vehicles /hour/lane  (3) 3000 vehicles /hour/lane  (4) 300 vehicles /hour/lane
18)	In a flexible pavement, the different materials with the CBR values are available as follows: 80%, 60%, 15% and 4%. Indicate the order (top to bottom) in which the materials are to be placed for making a good pavement (1) 4%, 15%, 60%, 80% (2) 4%, 60%, 80%, 15% (3) 80%, 60%, 15%, 4%. (4) 4%, 80%, 15%, 60%
19)	Calculate the Equivalent Axle load factor (EALF) for single axle load of 10 tons using fourth power formula.  (1) 2.26 (2) 22.60 (3) 1.23 (4) 11.23
20)	The hardness and toughness properties of a road aggregate will be obtained from
	(1) Aggregate Impact Test (2) Aggregate crushing test (3) Los Angeles Abrasion Test (4) Aggregate Shape test
21) noits	In the rigid pavement fatigue analysis, the ratio of flexural stress due to load and the flexural strength due to concrete is less than 0.45 indicates (1) the allowable number of repetitions of the axle loads is infinity (2) the allowable number of repetitions of the axle loads is zero (3) the allowable number of repetitions of the axle loads is 4500 (4) the allowable number of repetitions of the axle loads is 45000
22)	The gauge widths (in meter) for broad, standard and narrow gauges respectively are
	(1) 1.767, 1.650 and 0.760 (2) 1.676, 1.500 and 0.676 (3) 1.676, 1.435 and 0.762 (4) 1.876, 1.656 and 0.800
23)	For airports serving big aircrafts, ICAO recommends the cross wind component should not exceed  (1) 25 kmph (2) 35 kmph (3) 15 kmph (4) 23 kmph
24)	The monthly mean of average daily temperature for the hottest month of the year is 30°C and the monthly mean of the maximum daily temperature for the same month of the year is 45°C, Find out the airport reference temperature  (1) 25°C  (2) 35°C  (3) 40°C  (4) 15°C

25)	The minimum value of Composite Slee railways for track sleeper is	eper	Index (CSI) pr	rescribed on Indian
•	(1) 783 (2) 1352	(3)	1455	(4) 873
26)	The formula used for calculation  ( take G = gauge i radius of curve in meters)		V C	
	(1) $\frac{GV^2}{127R} m$ (2) $\frac{GV^2}{127R} cm$	(3)	$\frac{GV^3}{127R}m$	$(4)  \frac{GV^2}{225R} m$
27)	Cornice and coping is measured in _		356, 456, 456	
	<ol> <li>Running meter &amp; Square meter</li> <li>Square meter &amp; Square meter</li> </ol>			r & Running meter & Running meter
28)	In Simpson's formula for areas calculordinates is considered as	latio	n, the line join	ning the top of the
paigr	(1) elliptical (2) circular	(3)	parabolic	(4) straight
29)	To obtain the correct volume using the trashould always be (1) Multiplied (2) Added			rismoidal correction  (4) Zero
20)		TE I		abelied Riving
30)	In earth work excavation, normally lea of road estimate	d an	d lift is conside	ered for preparation
	(1) 30 m and 1.5 m	(2)	20 m and 1.5 i	m
	(3) 20 m and 2.0 m	(4)	30 m and 2.0 i	mile sale (E)
31)	The unconfined compressive strength of state was found to be 180 kN/sqm as Sensitivity, the soil may be classified a	nd 1		
			Sensitive	
	(3) Quick Clays	(4)	Extra Sensitive	e Clays
32)	If $R_1$ and $R_2$ are the radii of curvature orthogonal planes, the capillary rise is (1) $h_c = (\sigma / Y_W) \{R_1 + R_2\}$ (3) $h_c = (\sigma / Y_W) \{R_1 \times R_2\}$	give	n by:	
33)	The coefficient of permeability of a so	oil sa	mple having its	s void ratio as 0.50
	and co-efficient of percolation as 3.00			O Destroled
			$1.50 \times 10^{-4}$ cm $1.00 \times 10^{-4}$ cm	
	131 0.00 10 0111/5	1 -+ 1	1 1/1/1 \circ	1/ 5

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=0	4		
5(1)	1	1	A
20		1	H

\$(0)	layer to ensure the designated compaction energy is: (1) 56 (2) 25 (3) 35 (4) 50	
35)	The magnitude of total primary consolidation settlement of a 6m thick with single drainage was estimated as 96 mm. Later it was found that medium has double drainage. Then, the magnitude of total princonsolidation settlement will be:  (1) 48 mm  (2) 192 mm  (3) 384 mm  (4) 96 mm	t, the
36)	The shear strength of a pure clay specimen when tested in Unconcompression Test was found to be 100 kPa. If the same specimen was to in Tri-axial Compression Test, the deviatoric stress at which specimen undergo failure when the confining stress was 50 kPa, will be:  (1) 50 kPa  (2) 100 kPa  (3) 200 kPa  (4) 400 kPa	ested will
37)	A 3 m high retaining wall with vertical face is resisting a moist back fill horizontal top surface having $Y=20 \text{ kN/cum}$ . The percentage increase Total Active Earth Pressure, if the back fill gets submerged with $Y_{sat}=22 \text{ cum}$ and $Y_{w}=10 \text{ kN/cum}$ , is:  (1) 20 (2) 40 (3) 60 (4) 100	se in
38)	The factor of safety of a slope of given inclination of 6 m high construsing a soil with c=60 kPa, y=20 kN/cum and its Taylor's stall number=0.20, will be:  (1) 2.50 (2) 5.00 (3) 1.00 (4) 2.00	
39)	According to Boussinesque's theory, under the application of a point lo 100 kN on the surface, the pressure bulb corresponding to an increme vertical stress of 47.75 kN/sqm will extend to a depth of:  (1) 0.50 m  (2) 1.00 m  (3) 2.00 m  (4) 4.00 m	
40)	The ultimate bearing capacity of a shallow foundation laid on a cohesion soil medium was estimated as 200 kN/sqm, when the water table was below. All other conditions remaining the same, the ultimate bearing cap of the foundation, when the water table was risen to ground level, is:  (1) 400 kN/sqm (2) 200 kN/sqm (3) 100 kN/sqm (4) 50 kN/sqm	as far pacity
	9	P.T.O.

34) In a Laboratory, to perform IS Heavy Compaction Test, it was required to

use a mould of 1400 cc capacity in place of standard 1000 cc capacity mould. All other parameters remaining same, the number of blows to be applied per

41)	Ass	suming atancy	g corre	ection f s requir	for overed, the	rburd correc	en is	not re N-value	quired is:	eporte and	d as 1 correc	<b>501/A</b> 0/15/20 tion for
	(1)	45		(2)	35		(3)	25		(4)	15	
42)	situ	e, the	ar pile capaci	is estin	nated a e Pile it	s 100 f the d	kN. A iamet	end bea All other ter is do 300 kl	r paran ubled, N	neters is: (4)	remai	κN
43)	Wh	ich of	the fo	llowing	type o	of Pile	s is r	nore ap			found	ation of
	stru	ctures	constr	ructed o	n Expa	nsive	Clays	S:				
	(1)	Batte	r Piles	ei Juşahy			(2)	Sheet	Piles			
	(3)	Unde	er-rean	ned Pile	S		(4)	Comp	action	Piles		noo .
44)	The	type o	of Cais	sons pro	eferred	in site						xists, is:
	(1)	Pneu	matic	Caisson	IS		(2)	Open	Caisso	ns		1000, 10.
			Caisso									caissons
45)	Coff	fer Da	ms are	I he pe								
1467	(1)				es mea	nt for	stora	ge of wa	ater			
	(2)							s check				
	(3)		orary							e to cu	ire fou	indation
	(4)	Temp	orary	structi of fou	ures bu	uild to	res	erve w	ater o	utside	to fa	cilitate
46)	The			for Kine			sity is		(2			
	(1)	FL-27	e Carrie	(2)	ML-17	r-1 arche ve	(3)	$L^2T^{-2}$		(4)	L2T-1	
47)	corre	stream espond x <sup>2</sup> +y <sup>2</sup>	ding po	otential	or a po functio 2xy	n (Φ),	assui	w field ming ze x²-y²	is giv ro pote	ential a	$\psi = x^2$ at the contact $x-y$	-y², the origin is
48)	pres	sures 1	vs throneasur m/min	ed by a	large si pitot tu	ize pi <sub>l</sub> be are	pe. T. e 0.3n	he stag 1 and 0.	nation 24m of	press	ure an	d static velocity
. 10	(1)	1.08	d barr	(2)	65.00		(3)	10.8		(4)	0.65	

49)	sec at a depth of 0.20m. T				IISCII	inge of 1.6iii	1
•	(1) 1.03m (2)				(4)	2.0m	
50)	In a catchment there are so of 92.8cm and standard gauge stations is 30.7cm. mean rainfall, the optimum	deviation of For a 10%	f the degre	rainfall is reco	orded he me	in these rai	n
	(1) 5 nos (2)					11 nos	
51)	The area between the two 55cm and 65cm is 150km over the basin of 250km <sup>2</sup>						
10fil	(1) 50cm (2)	52cm	(3)	56cm	(4)	60cm	
52)	The total observed runoff of 2.8cm/hr is 25.2×10 <sup>6</sup> m infiltration rate for the bas	n <sup>3</sup> from a bas	ing a	4hr storm with 280 km <sup>2</sup> area.	a uni What	form intensit is the averag	y ge
			(3)	5.2mm/hr	(4)	5.5mm/hr	
53)	A volume of 3×10 <sup>6</sup> m <sup>3</sup> of aquifer, uniformly over ar from initial level of 102m (1) 0.20 (2)	area of 5km	n <sup>2</sup> . The spec	e pumping low	ered t e aqu	the water tab	
54)	A hyetograph is a plot of (1) Cumulative rainfall V (3.) Rainfall depth Vs du	Vs time	(2) (4)	Rainfall intens Discharge Vs		s time	
55)	The unit of intrinsic perm (1) cm/day (2)		(3)	Darcy/day	(4)	cm <sup>2</sup>	
56)	If $S_y$ = specific yield and (1) $S_y + S_r$ = void ratio (3) $S_y + S_r$ = 1.0	S <sub>r</sub> = specific	(2)	ntion then $S_y + S_r = poros$ $S_y + S_r = perm$	sity eabil	ity	
57)	If duty (D) is 1428 hecta irrigated crop, then delta (1) 102.8 (2)					days for an	

58)	Which one of the following equations represents the downstream profile of Ogee spillway with vertical upstream face? (X, Y) are the coordinates of the point on the downstream profile with origin at the crest of the spillway and H <sub>d</sub> is the design head.
likin Dia t	(1) Y/H = $-0.5(X/H_d)^{1.85}$ (2) Y/H <sub>d</sub> = $-0.5(X/H_d)^{1/185}$ (3) Y/H <sub>d</sub> = $-2.0(X/H_d)^{1/185}$ (4) Y/H <sub>d</sub> = $-2.0(X/H_d)^{1/185}$
59)	For no tension to develop in the gravity dam the eccentricity 'e' of the resultant force should be  (1) Less than b/ <sub>3</sub> (2) Less than b/ <sub>6</sub>
	(1) Less than $\frac{b}{6}$ (2) Less than $\frac{b}{6}$ (3) Less than $\frac{b}{12}$
60)	Lacey's equations can be used for the design of
	(1) Unlined channels only (2) Lined channels only
	(3) Both lined and unlined channels (4) Neither lined nor unlined channels
	Syphon aqueduct is a cross drainage work provided to carry canal over a natural drain when  (1) Canal bed is well above the HFL of the natural drain  (2) Canal bed is at the same level as the bed of the natural drain  (3) Canal bed is below the HFL of the natural drain  (4) Canal bed is below bed of the natural drain
62)	Poise is the CGS unit of (1) Kinematic viscosity (2) Dynamic viscosity (3) Mass Density (4) Weight Density
63)	The velocity gradient is 1000/s. The viscosity is $1.2\times10^{-4}$ N-s/m². The shear stress is (1) $0.12\text{N/m}^2$ (2) $1.2\times10^{-7}$ N/m² (3) $12\text{N/m}^2$ (4) $12\times10^{-5}$ N/m²
64)	If Z is measured vertically upwards, dp is given by (1) $dp = ydz$ (2) $dp = \rho dz$ (3) $dp = -ydz$ (4) $dp = -\rho Dz$
65)	If $\psi = 3x^2y - y^3$ , the values of u and v are (1) $6xy$ , $3x^2 - 3y^2$ (2) $3x^2 - 3y^2$ , $6xy$ (3) $(3x^2 - 3y^2)$ , $-6xy$ (4) $3y^2 - 3x^2$ , $6xy$
66)	In a three dimensional motion of a fluid, the component of rotation about the

x-axis, ω<sub>x</sub> is

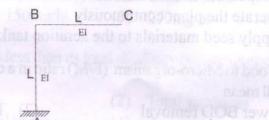
$$(1) \quad \frac{1}{2} \left( \frac{\partial w}{\partial y} - \frac{\partial v}{\partial z} \right) \quad (2) \qquad \frac{1}{2} \left( \frac{\partial u}{\partial z} - \frac{\partial w}{\partial y} \right) \quad (3) \quad \frac{1}{2} \left( \frac{\partial v}{\partial x} - \frac{\partial u}{\partial y} \right) \quad (4) \quad \frac{1}{2} \left( \frac{\partial v}{\partial x} - \frac{\partial w}{\partial y} \right)$$

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67)	Existence of velocity potential implies t	hat	
	(1) Fluid is in continuum	(2)	Fluid is irrotational
	(3) Fluid is ideal	(4)	Fluid is compressible
68)	Piezometric head is the summation of		
	(1) Velocity head and elevation head	(2)	Velocity head and pressure head
	(3) Pressure head and elevation head	(4)	Total head
69)	The Bernoulli's equation is written with	th usu	al notation as $p/y + v^2/2g + z =$
	Constant. In this equation each of the t		
	(1) Energy in kg.m/kg mass of fluid	(2)	Energy in Nm/kg mass of fluid
	(3) Energy in Nm/N weight of fluid	(4)	Energy in KW/kg mass of fluid
70)	A 50mm diameter jet having a velocity	of 25r	n/s strikes a flat stationary plate,
	the normal of which is inclined at 60° t	o the	axis of the jet. The normal force
	exerted on the plate in Newtons is		
	(1) 460N (2) 360 N	(3)	640 N (4) 630 N
71)	Reynolds number of a flow is the ratio	of	
1	(1) Gravity forces to viscous forces	(2)	Gravity forces to pressure forces
	(3) Inertia forces to viscous forces	(4)	Viscous forces to pressure forces
72)	Ratio of the average velocity to maxim circular pipe is	um ve	elocity for steady laminar flow in
=0.01	(1) 1/2 (2) 2/3	(3)	3/2 (4) 2
73)	Difference in elevation between TEL and	d HGI	of pipe flow at a point is equal to
	(1) Datum head	(2)	Velocity head
	(3) Pressure head	(4)	Piezometric head
74)	Boundary layer separation takes place	when	
,	(1) $\partial p/\partial x > 0$		∂v/∂y≤0
	(3) $\partial p/\partial x > 0 \& \partial v/\partial y \le 0$	(4)	$\partial \mathbf{p}/\partial \mathbf{x} < 0$
75)	For a uniform flow with a depth of or rectangular channel, the specific energy		
	(1) 2.4m (2) 0.8m		2.6m (4) 1.8m
70	A		membratist so right instant our state.
76)	A water turbine of expected efficiency of 10m <sup>3</sup> /s. The unit weight of water is MW is		
	(1) 3.06 (2) 3060	(3)	3600 (4) 850.

77)	A sedimentation tank 6m wide, 15m long of water. The surface overflow rate in 1			h is trea	ating 2MLD
	(1) 858 (2) 926	(3)	1028	(4)	748
78)	The efficiency of a sediment removal in not depend upon the	a con	tinuous sedin	entatio	n tank does
	(1) Discharge through the tank	(2)	Width of the	tank	
	(3) Length of the tank	(4)	Depth of the	tank.	
79)	The disinfection efficiency of chlorine i	n wat	er treatment		
	(1) Is not dependant on p <sup>H</sup> value	(2)	Is increased by	increas	e in pH value
	(3) Remains constant at all p <sup>H</sup> values	(4)	Is reduced by	increase	e in p <sup>H</sup> value.
80)	A 2% solution of a sewage sample is 20°C. If initial DO and final DO values 8.5 mg/lit and 5.5 mg/lit respectively, the	after	5 days of inc	ubation	
- ,	(1) 50mg/lit (2) 150mg/lit	(3)	250mg/lit	(4)	350mg/lit
81)	If total hardness of water is less than its tot will be equal to (1) Total alkalinity	al alk	alinity the non- Total hardne		ate hardness
	(3) Total alkalinity - total hardness	(4)	Zero		
82)	If for diluting 25ml of water sample 175 added to make the water sample to just le Number (FTN) will be (1) 6 (2) 7			Flavou	
83)	In network of pipes  (1) The algebraic sum of discharges at (2) The algebraic sum of head losses  (3) The elevation of hydraulic grade life  (4) Elementary circuits are replaced by	aroun ne is	nd each circuit assumed for e	is zero	
84)	The dispersion of pollutants in atmosph (1) Environmental lapse rate is greated (2) Environmental lapse rate is less th (3) Environmental lapse rate is equal to (4) Maximum mixing depth is equal to	r than an adi	adiabatic laps iabatic lapse r abatic lapse ra	se rate ate	

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85)	Which one of the following pollutant (or) pairs of pollutants is formed due to photochemical reactions
	(1) CO alone (2) O <sub>3</sub> and PAN (3) PAN and NH <sub>3</sub> (4) NH <sub>3</sub> and CO
86)	If a sewer carrying a discharge of 3 cumecs outfalls into a river having a discharge of 10 cumecs and DO equal to 9.1mg/lt, the resultant DO of the mix will be equal to  (1) 5mg/lt  (2) 6mg/lt  (3) 7mg/lt  (4) 8mg/lt
87)	The natural process under which the flowing river gets cleaned is known as  (1) Oxidation (2) Photosynthesis  (3) Reduction (4) Self-purification
88)	Recirculation in activated sludge process is done to  (1) Dilute the incoming sewage  (2) Dampen the effect of the flow variation  (3) Operate the plant continuously  (4) Supply seed materials to the aeration tank
89)	Lower Food to Micro-organism (F/M) ratio in a conventional activated treatment plant will mean  (1) Lower BOD removal  (2) Higher BOD removal  (3) No effect on BOD removal  (4) Medium BOD removal
90) M	The relative stability of a sewage sample whose dissolved oxygen levels equals the total oxygen required to satisfy its BOD is  (1) Zero (2) 1% (3) 100% (4) Infinity
	A rectangular box made with thin uniform plate measures 2000mm x 1000mm x 1000mm. When the box is subjected to certain internal pressure, the dimensions in respective directions have changed by $+2.0$ mm, $-1$ mm and $+1$ mm. The change in the volume of the box is  (1) $2.0 \times 10^6$ mm³ (2) $1.0 \times 10^6$ mm³ (3) $2.0 \times 10^3$ mm³ (4) $2.0 \times 10^5$ mm³
92)	A rectangular section of a beam is acted upon by certain amounts of shear force and bending moment. Whereas the shear stress varies, the variation of bending stress is along the depth.  (1) linear with zero value at centroid, linear with zero value at centroid  (2) parabolic with zero value at centroid, linear with zero value at centroid.  (3) parabolic, with zero value at top & bottom, linear with zero value at middle.  (4) linear with zero value at centroid, parabolic with zero value at top & bottom.

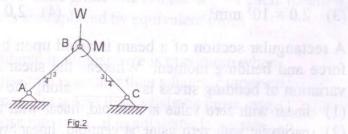
- 93) A simply supported beam of span L, carries a UVL with intensity varying from w/unit downward at left support to w/unit upward at right support. The reactions at left and right supports are respectively:
  - (1) wL/4 upward and wL/4 downward
  - (2) wL/4 downward and wL/4 upward
  - (3) wL/6 upward and wL/6 downward
  - (4) wL/8 upward and wL/8 downward
- 94) A cantilever beam of span L, uniform flexural rigidity EI is subjected to a unit couple at its free end. The deflection at the centre of the beam is:
  - (1)  $L^2/2EI$
- (2)  $L^{2}/8EI$
- (3)  $L^{2}/4EI$
- (4) L<sup>2</sup>/16EI
- 95) For the L bent shown in Fig. 1, the flexural rigidity of both arms AB and BC is El carries a vertical downward load W at C. The deflection and rotation at B (neglecting axial deformations) are and



- (1)  $WL^2/2EI \rightarrow WL/EI$
- Fig.1
- (2) 2WL³/EI↓,WL²/3EI

(3) WL³/EI↓,WL²/EI

- (4)  $WL^2/2EI \rightarrow WL^2/EI$
- 96) A simply supported beam of span L, carries two couples of magnitude M each acting at both middle third locations of the beam. While one of them is acting clockwise the other is acting counter clockwise. Magnitude of the maximum shear force acting in the beam
  - (1) 2M/L (2) M/L
- (3) 1.5M/L 4) 0
- 97) The plane truss shown in Fig.2 carries a point load W and a moment M at the location B. Force carried by member AB is



- (1) 5W/6 (compressive) (2) W/2 (compressive)
- (3) 5W/6 + M/L (compressive) (4) W/2 M/2 (compressive)

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		lysis of trusses is		follow	ring assur	mptions.	The correct
		bination of assump	1.1	0.00			
	(i)	All loads act at joi					
	(11)	All joints are smoo		-			
	(iii)	Truss is made with					
	(iv)	The axis of all me				weights are	ignored.
TIGHT	(1)	i, ii and iv (2)	i, ii, iii	(3)	i, iv	(4)	i, iii, iv
99)	The	correct combination	n of conditions	that d	efines a ri	gid joint is	cutagib <sub>eer</sub>
	i)	All members meet				BAIVE UC	Situas of
	ii)	All deflections and	d rotations at the	e joint	are zero.		We dimen
	iii)	All members meet	ing at the joint i	inderg	go same d	efections a	t that joint.
	iv)	All members meet					behalf the property of the control
	(1)	i, ii (2)	i, iii	(3)	ii, iv	(4)	iii, iv
			(a) (b) (c) (d)				A (E) min
		econd degree pagal	herr forçalis vis				
		anamont smbm d le	CHORESTER MI	777			
			Fig.3	//			
			<u>rig.s</u>				
			Tig.5	dered (3)			
		6 (2)	are nulls (44 slo	(3)	8		
		6 (2)	through a 11 to	(3)	8	Fot Poolis	
01)	In th	6 (2) ne moment distribu	11 tion method of	(3) plane	frame an	alysis, the	distribution
01)	In th	6 (2) ne moment distributor for a member at a	tion method of joint depends or	(3) plane	frame an con	alysis, the	distribution f conditions.
01)	In the	6 (2) ne moment distribu	tion method of joint depends on atio of all the me	(3) plane n_embers	frame an cons meeting	alysis, the abination of at the joint	distribution f conditions.

(2) i, iii

(1) ii, iii

iv) Support conditions at the farther ends of members meeting at the joint.

(3) iii, iv

(4) i, iv

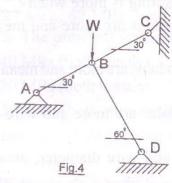
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102) A cantilever beam of uniform flex	ural rigidity with span L and depth D is
subjected to temperatures T. on th	e upper face and T <sub>2</sub> on the lower face. If
$T_{1} < T_{2}$ and $\alpha$ is the coefficient of	of linear expansion for the material, the
deflection at the free end of the bea	om is
(1) $\alpha \left( \frac{1}{2} - \frac{1}{1} \right) L / 2D $ (upward)	(2) $\alpha (T_2 - T_1) L^2/D$ (upward)
(3) $\alpha (1_2 - 1_1) L /2D$ (upward)	(4) $\alpha (T_2 - T_1) L^2 / 2D$ (downward)
(3) i.iv (4) i.ii.tv	
directions are 120 MPa, 40 MPa (	al stresses in two mutually perpendicular both tensile) associated with a tangential
stress of 30 MPa. The principal str	esses at the location are
(1) 120 MPa, 40 MPa (both tensile	
(2) 130 MPa, 30 MPa (both tensile	
(3) 130 MPa, 30 MPa (both comp	rescive)
''	
(4) 130 MPa (tensile), 30 MPa (c	ompressive)
10.00	
deformation of the body in the direction of t	from a height of h. If the instantaneous ction of weight is $\delta$ , the work done by the
(1) $\frac{1}{2}$ W (h + $\delta$ )	(2) $W(h + \delta)$
$(3) \frac{1}{2} W h + W \delta$	(4) $W h + \frac{1}{2} W \delta$
(3) /2 ** 11 * ** 0	(4) W II + 72 W 0
105) In a hearn where the varieties of al-	11
105) In a beam, where the variation of she	the variation of her discovered parabola and
the variation of loading is,	
	(2) linear, square parabola
(3) cubic parabola, square parabol	a (4) linear, cubic parabola
100 A farm of	1 1
106) A force of magnitude 5 N moves the	rough a distance of 4mm in a direction,
the force is	rce. The magnitude of the work done by
(1) 10/2 N	101) In the moment distribution methodic
	(3) 10 N.mm (4) 20 N.mm
members meeting at the joint.	The slenderness (and of all the
10/) The shear centre for an angle section	n is located
	(2) at the intersection of flanges
(3) at the centroid of the angle	(4) at the rigidity centre of the angle

108) A beam has a circular cross section. If the plane of loading on the beam does not coincide with the centroidal axis of the beam, the member is subjected to
(1) axial force, shear force and bending moment
(2) torque, shear force
(3) torque, shear force and bending moment
(4) axial force, shear force and torque
109) The stiffness of a close coiled spring is more when
(1) wire diameter, rigidity modulus are more and mean radius, number of turns are lesser
(2) number of turns, rigidity modulus are more and mean radius, wire diameter are lesser
(3) mean radius, rigidity modulus are more and wire diameter, number of turns are lesser
(4) rigidity modulus is more and wire diameter, mean radius, number of turns are lesser
<ul> <li>110) A pressure vessel in the form of a thin cylinder of 1m diameter and 1mm plate thickness is subjected to an internal fluid pressure of 0.2 MPa. The maximum shear stress in the material is</li> <li>(1) 50 MPa</li> <li>(2) 0 MPa</li> <li>(3) 25 MPa</li> <li>(4) 37.5 MPa</li> </ul>
<ul> <li>111) Two circular shafts of same length, weight and material are compared for strength. The first one is a solid shaft and the other is a hollow shaft of outer to inner diameter ratio as 2. The ratio of the torque carrying capacity of the hollow shaft to solid shaft considering the shear stress criterion alone is</li> <li>(1) 2.5/√3</li> <li>(2) 5√3/6</li> <li>(3) √3</li> <li>(4) √3/2</li> </ul>
112) For a circular cross section subjected to shear force, the ratio of maximum shear stress to average shear stress is  (1) 1.0 (2) 2.0 (3) 1.5 (4) 4/3
<ul> <li>113) A cantilever beam of span L and flexural rigidity EI carries a point load W, vertically downwards at its free end. The free end of the beam is resting at the centre of another simply supported beam of span L and flexural rigidity EI. The support reactions for the simply supported beam are and</li> <li>(1) W/4, W/4</li> <li>(2) W/2, W/2</li> <li>(3) 8W/17, 8W/17</li> <li>(4) 8W/3, 8W/3</li> </ul>

- 114) A simply supported beam of span L and flexural rigidity EI carries a UDL of intensity w/unit run all along its span. The beam is supported at its centre by a linear spring of stiffness k. The force carried by the spring is
  - (1)  $5kwL^4/(kL^3+8EI)$
- (2)  $5kwL^4/8(kL^3+48EI)$

(3)  $5wL^4/8(kL^3+8EI)$ 

- (4)  $5wL^4/8(kL^3+48EI)$
- 115) The three member plane truss A-B-C-D, shown in Fig.4 supports a vertical load W at B. The magnitude of the force carried by member BD is

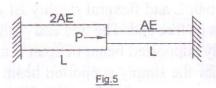


- (1) W (compressive)
- (2) 0.5 W (compressive)
- (3)  $(\sqrt{3}/2)$  W (compressive)
- (4) 0.5W (tensile)
- 116) The symmetry of flexibility matrix is due to
  - (1) Betty's theorem

(2) Maxwell's reciprocal theorem

(3) Eddy's theorem

- (4) Castigliono's theorem
- 117) A fixed beam of span L and uniform flexural rigidity EI carries a vertical downward load W at its mid span. If a hinge is introduced in the beam at the location of the load, the deflection under the load is
  - (1) WL<sup>3</sup>/12EI
- (2) WL<sup>3</sup>/24EI (3) WL<sup>3</sup>/16EI
- (4) WL3/48EI
- 118) A stepped bar A-B-C of total length 2L carries an axial load P at B as shown in Fig.5. Axial rigidity of segment AB is 2AE and that of BC is AE. The displacement at B is ...



- (1) PL/3AE (2) 0
- (3) PL/2AE
- (4) PL/AE

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119) The	static indeterminacy	y for the conti	nuou	s beam shown	in Fig.6 is
ined in		Д	SI (S.	one i lange	- Constitution in
		17771			al Properties
onthrea		Fig.6			
(1)	6 (2)	3	(3)	2	(4) 4
of i		he magnitude	e of a	absolute maxii	m long UDL segment mum moment in the (4) 88.75kN.m
TIONE IT	A nation and should				n Systems and wife?
acti (1) (2)	ng on full span who intensity is larger to moment diagram is loads on the span.	han all other less above the be	oads	acting on the sign moment orders	dinates caused by all oment caused by all
a pr	re-stressing force of	magnitude Pris of the mem	der.	in eccentricity The displacem	gidity EI is subjected e below the neutral ent of the beam at a
(1)	PeL <sup>2</sup> /4EI				
(3)	PeL <sup>2</sup> /2EI		(4)	PeL <sup>2</sup> /EI	support resenous
	a member subjecte ermined in such a w				lent joint loads are
(1)		actions cause	d by	equivalent join	nt loads are same as
(2)	Pine of the leading of the latest and the leading of the latest and the latest an	loads cause s	ame	set of nodal dis	placements as caused
in (3)	The equivalent jo	int loads to	gethe	r with member	er loads ensure the
(4)		oint loads tog			er loads ensure the

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124) 4.	outhorned atmentions is an audich of	501/A
	orthogrid structure is one which sanditions	atisfies following combinations of
i)	A two dimensional framed structure different directions.	e consisting of members oriented in
ii)	Loading plane perpendicular to the	plane of structure.
iii)	Member end actions are the axial moments.	
iv)	Angle between members is always 9	
v)		
(1)		of intensity (0k) wising in it, ig.
(3)	ii, iii, iv, v (4)	i, ii, iv, v
125)In t	he stiffness matrix method, the bound	dary conditions are needed to avert
$\overline{(1)}$	divergent solution (2)	singularity
(3)	irrational solution (4)	
are i) ii) iii) iv) (1)	he direct element method of structural an needed for following combinations of They facilitate the transformation of matrices to global stiffness and global They are useful for determining the They facilitate the conversion of remember oriented displacement matrathe are useful for determining meministry, iii (2) ii, iii (3)	of actions
, at s	6ΕΙ α /L², 4ΕΙ α /L,- 6ΕΙ α /L², 2ΕΙ 6ΕΙ α /L², 2ΕΙ α /L,- 6ΕΙ α /L², 4ΕΙ	e by α radians anti clockwise. The rts (assuming upward displacement respectively are α /L I α /L I α /L
meres The rota (1)	ing slope deflection method, the end in mber 4m long with flexural rigidity EI pectively. The beam carries a 80kN verse support moments at A and B are eations as positive) taking EI = 5 x10 <sup>4</sup> +30 kN.m, -15 kN.m (2) -7.5 kN.m, +15 kN.m	are found as EI/1000 and – EI/2000 ertical downward load of 1m from A (assuming clockwise moments and kN.m are respectively.  +45 kN.m, +15 kN.m

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uniform
re fived

129	flexi	ee members AB, BC and BD 3m, ural rigidity are rigidly connected at le at D it is hinged the distribution fare joint B are respective	B. If ctors	the supports at A and C are fixed	
	(1)			4/11, 3/11, 4/11	
	(3)		/	1/3, 1/3, 1/3	
130	bear	three hinged parabolic arch, the thin moment coinciding with H-mome	ent in	ndicates	
	(1)	Arch all along carries only radial sl			
	(2)	Arch all along carries only beam march all along carries only normal			
	(4)	Arch all along carries only H- mon			
131	The	able carrying a load of w/unit run is supports are at the same level and at tensions in the cable are			
gil n			(2)	$\sqrt{((wL/2)^2+(wL^2/8h))}$ , wL <sup>2</sup> /16h	
	(3)	$\sqrt{((wL/2)^2+(wL^2/8h))}, wL^2/8h$	(4)	$\sqrt{((wL/2)^2+(wL^2/2h))}$ , wL <sup>2</sup> /8h	
132	,	antilever beam AB is fixed at left en jugate beam will supports			
	(1)	fixed at A and free at B	(2)	free at A and simple support at B	
	(3)	free at A and fixed at B	(4)	simple supports at both A and B	
133		steel tension member, it's maximum ted connections is governed by	stren	gth in case of adequately designed 	
5	(1)	The slenderness of the member			
	(2)	The tensile strength of net section	al are	ea of the member.	
HEO	(3)	The strength of the bolted connec	tion.		
	(4)	The section modulus of the memb	per		
134	() The	e principal reason for adding stiffer	ners 1	to the web of a steel beam is to	
	(1)	Increase its moment carrying capa	acity.	nased date zasnik grab za da da sa d	
	(2)	Reduce the deflection of the bean	n.		
	(3)	Increase the stiffness of the web.			
	(4)	Improve aesthetics.			
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1	35) In th	built-up steel columns, the lacing is provided.
5	i)	to keep all individual sections together.
	ii)	to take-up lateral shear to an extent of 2.5% of axial force.
	iii)	to increase the bending strength of the section.
	iv)	to increase the compressive strength of the section.
	(1)	i , iii (2) ii , iii (3) iii , iv (4) i , ii
	. 1700	130) in a three hinged parabolic aleb. the third image follogated at the
1	36) The	ist of principal components of a plate girder is
	i)	Top and bottom flange plates and web plates. To land the dark (S)
	ii)	Horizontal stiffeners, Intermediate and bearing stiffeners.
	iii)	Cleat angles and seat angels.
	iv)	Web splicing and flange splicing.
	(1)	i , iii, iv (2) i, ii, iii (3) i, ii, iv (4) ii, iii, iv
1	37) The	requirements to be satisfied in the design of a gantry girder are that it ha
	to v	thstand effects due to
	(1)	Moving loads, impact effects and fatigue.
	(2)	Moving loads, longitudinal loads, lateral loads, impact effects and fatigue
	(3)	Lateral loads, longitudinal loads, impact effects and fatigue.
	(4)	Dynamic loads, longitudinal loads, impact effects and fatigue.
1	38) The	Lug angle is a member which
	10-12-017-17	is connected to the main tension member to transfer the tensile force economically to the joint.
	(2)	is connected to the main tension member for erection purpose.
	(3)	is connected to the main tension member to increase its strength locally
	(4)	is connected to the main tension member to increase its stiffness
1	39) The	expression working out the thickness of slab base is given by
	If t ben	thickness of slab base, $w = pressure$ under slab base, $\sigma_{bs} = permissible ing stress in slab base, a, b = longer and shorter projections of the slab edge to the column member, Poisons ratio = 0.25.$
		$t = ((3w/\sigma_{bs})(a-b/4))^{1/2}$ (2) $t = ((3w/\sigma_{bs})(a^2-b^2/4))^{1/2}$
		$t = ((3w/\sigma_{bs})(a^2-b^2))^{1/2} $ (4) $t = ((3w/\sigma_{bs})(a-b^2/4))^{1/2}$

and d	0	1	A
Atom III	3 1	1	/B
- 79	7 0	1	

140) A fixed beam of span L carries a UDL of intensity w/unit run. If the plass moment of the beam section is M <sub>p</sub> , collapse occurs when number	
plastic hinges are formed and wL =	
(1) 2 and $8M_p/L$ (2) 3 and $8M_p/L$	
(3) 3 and $16M_p/L$ (4) 2 and $16M_p/L$	
141) A cantilever beam of span L carries a point load W at its free end. Plas moment of the section from support to middle of span is 1.5 $M_p$ and fr there to free end is $M_p$ . Collapse occurs when $W = \underline{\hspace{1cm}}$	
(1) $1.5M_p/L$ (2) $M_p/L$ (3) $2M_p/L$ (4) $0.75M_p$	/L
142) According to IS: 456-2000, in the design of isolated RCC column foot using RCC with an effective depth d, the critical sections to be checked	ing are
(1) Bending moment at the face of the column, one-way shear at d/2 av from the face of column and two-way shear at d around the column.	vay
(2) Bending moment at the face of the column, one-way shear at d av from the face of column and two-way shear at d around the column	vay
(3) Bending moment at d/2 away from the face of the column, one-values shear at d away from the face of column and two-way shear at around the column.	vay
(4) Bending moment at the face of the column, one-way shear at d average from the face of column and two-way shear at d/2 around the column	vay m.
143) Minimum amount of high strength deformed bar reinforcement used in so slabs shall not be less than of the total cross sectional area the slab according to IS: 456-2000.	olid a of
(1) 0.15% (2) 0.12% (3) 0.20% (4) 0.25%	
144) When the depth of the web of beam is more than mm, side for reinforcement of % of the web area is needed to be provided accord to IS:456-2000.	ding
(1) 600, 0.1% (2) 750, 0.15% (3) 600, 0.15% (4) 750, 0	.1%
145) Maximum spacing of vertical shear reinforcement measured along the axt the RCC beam shall not exceed, whichever is lowest.	is of
(1) 0.75d or 300mm (2) 0.75d or 400mm	
(3) 0.50d or 250mm (4) 0.5d or 300mm	
	TO

501/A 146) The bearing stress check for column footing in limit state design specifies that
the value subjected to a maximum of 2 multiplied by bearing stress
shall be more than the compressive stress at the base of the column
$\Lambda_1$ = supporting area for bearing of footing, which in sloped or stepped
footing maybe taken as the area of the lower base of the largest frustum of a
pyramid or cone contained wholly within the footing and having for its upper
base, the area actually loaded and having side slope of one vertical to two
horizontal; and, $A_2$ = loaded area at the column base.
(1) $\sqrt{(A_1/A_2)}$ , 0.30 $f_{ck}$ (2) $\sqrt{(A_1/2A_2)}$ , 0.45 $f_{ck}$
(3) $\sqrt{(A_1/A_2)}$ , 0.45 $f_{ck}$ (4) $\sqrt{(A_1/A_2)}$ , 0.60 $f_{ck}$
1 2 CK
147) The functionality of a wall, retaining wall and a shear wall in order is
i) resist predominantly vertical loads
ii) resist lateral loads perpendicular to the plane of wall
iii) resist lateral loads in the plane of wall
(1) ii, iii, i (2) i, ii, iii (3) iii, ii, i (4) ii, i, iii
1400 G. 11 11 11 11 11 11 11 11 11 11 11 11 11
148) Splicing of reinforcement in flexure members is taken-up at a location where
bending moment is less than the moment of resistance at that section
and not more than of bars are spliced at any particular section.
(1) 75%, 50% (2) 50%, 75% (3) 25%, 50% (4) 50%, 50%
149) The loss stress due to creep in steel in a pre-stress problem is given by the
formula, where $\alpha$ = creep coefficient, $f_c$ = stress in concrete
$E_c = modulus of elasticity of concrete and E_s = modulus of elasticity of steel$
(1) $\alpha(f_c/E_c) E_s$ (2) $\alpha(f_c/2E_c) E_s$ (3) $\alpha(f_c/E_s) E_c$ (4) $2\alpha(f_c/E_c) E_s$
150) The principal reason for adopting pre-stressing cable profiles in flexure
members as parabolic is due to the fact that

- (1) They need to resist both bending moments and shear forces in members.
- (2) The strength of pre-stressing cables is maximum in parabolic shapes
- (3) The profile of moment caused by self weight of structure is parabolic - and to counter this, the cable profile also needs to be parabolic.
- (4) Its a regular practice

